

EXTENDING INTEGRAL CONCEPTS TO CURVED BRIDGE SYSTEMS

by

Saeed Eghtedar Doust

A DISSERTATION

Presented to the Faculty of

The Graduate College at the University of Nebraska

In Partial Fulfillment of Requirements

For the Degree of Doctor of Philosophy

Major: Interdepartmental Area of Engineering

(Civil Engineering)

Under the Supervision of Professors Elizabeth G. Jones and Atorod Azizinamini

Lincoln, Nebraska

November, 2011

UMI Number: 3487259

All rights reserved

INFORMATION TO ALL USERS

The quality of this reproduction is dependent on the quality of the copy submitted.

In the unlikely event that the author did not send a complete manuscript and there are missing pages, these will be noted. Also, if material had to be removed, a note will indicate the deletion.



UMI 3487259

Copyright 2011 by ProQuest LLC.

All rights reserved. This edition of the work is protected against unauthorized copying under Title 17, United States Code.



ProQuest LLC.
789 East Eisenhower Parkway
P.O. Box 1346
Ann Arbor, MI 48106 - 1346

EXTENDING INTEGRAL CONCEPTS TO CURVED BRIDGE SYSTEMS

Saeed Eghtedar Doust, Ph.D.

University of Nebraska, 2011

Advisors: Elizabeth Jones and Atorod Azizinamini

The behavior of integral abutment systems and the extension of their application to curved bridges are investigated. First, the stresses in the elements of a typical integral abutment are studied by conducting nonlinear finite element analysis using the software package Abaqus. The results are design recommendations for the details of such abutments. The effect of integral abutments on the responses of bridges is also investigated. Steel and concrete bridge systems are studied separately.

The studied steel bridge systems are composed of composite I-girder superstructures and integral abutments supported on steel H-piles. A series of finite element studies for different bridge lengths and radii are conducted and the effects of several load cases on the bridges are studied. In these bridges, the stresses in the abutment piles are of critical importance from the design standpoint. The results show that horizontal curvature mitigates these stresses. The bridge movement is also studied and a procedure to find the end displacements of curved bridges is presented. Pile orientation is another significant design factor that is studied elaborately. The results indicate that, for straight bridges, the strong-axis pile bending yields lower levels of stress. A method for finding the optimum pile orientation in curved integral bridges is developed. The effect of different bearing types is also investigated. This investigation reveals the superior structural performance of elastomeric bearings compared to other bearing types.

The concrete bridge systems that are studied consist of voided slab superstructures, integral abutments and concrete drilled shafts. A matrix of finite element studies is performed for different lengths and curvatures. Similar to steel I-girder bridges, it is concluded that horizontal curvature mitigates the internal forces of the abutment elements. The orientation of the concrete shafts is also examined which again shows the advantage of strong-axis orientation. Integral abutment bridges can have flexible piers integrally connected to the superstructure to eliminate all the bridge bearings. The effect of such integral piers on the internal forces of integral abutments is also examined. In these flexible piers, moment magnification can be of crucial significance. It is shown that choosing the integral abutment system reduces the magnification effects in the slender pier columns compared to jointed bridge systems.

© Copyright by

Saeed E. Doust

(2011)

Dedication

To
My wife Golboo
and
My daughter Taraneh

PREVIEW

Acknowledgement

The present dissertation was conducted in the National Bridge Research Organization under the supervision of Professor Atorod Azizinamini. I am really thankful of him for his guidance, advice and help throughout the course of my doctoral studies.

I would like to express my sincere thanks to Professor Andrzej Nowak for his valuable considerations during the past years. His attention has meant a lot to me. I am extremely appreciative of his continual kindness.

I am deeply grateful of Professor Mehrdad Negahban for his valuable guidance and friendship. He gave me confidence in the most difficult days that I had.

I wish to thank Professor Elizabeth Jones, the Graduate Chair of the department for her sincere helps and guidance. I also thank Professor Fred Choobineh who served on my committee and evaluated my studies. And I appreciate the friendship and assistance of Dr. Aaron Yakel during the past years. I will never forget his precious helps.

I want to offer my special thanks to my beloved wife, Golboo, for her priceless patience, understanding and support. If she was not helping me, I was not able to accomplish this work. I owe her a life. And I apologize my daughter, Taraneh, for those times that I was tired and couldn't play with her. I promise to spend more time for her from now on.

At the end, I would like to thank my parents for spending their lives on my growth and progress.

Table of Contents

Table of Contents	v
List of Figures.....	xi
List of Tables.....	xix
Chapter 1 Introduction, Background and Objectives	1
1.1 Introduction	1
1.2 Background.....	6
1.3 Literature Review	7
1.3.1 Straight IA Bridges	8
1.3.2 Curved Bridges	10
A) Analysis.....	14
B) Elastic lateral torsional buckling capacity	15
C) Cross-Frame Spacing.....	16
D) Effect of Cross Frames.....	16
E) Flange Local Buckling of Curved Girders.....	17
1.3.3 Curved IA Bridges.....	17
1.4 Scope	19
1.5 Objectives of the study	20
Chapter 2 Mechanistic Study of Curved IAB	23
2.1 Introduction	23
2.2 Analysis of a Curved Girder.....	24
2.3 Analysis of an I-Girder under Torsion	27
2.4 Analysis of Two-girder Bridges	32
2.5 Analysis of Multi-girder Bridges.....	37
2.6 Cross Frame Spacing.....	41
Chapter 3 Detailed Study of Connections in an Integral Abutment.....	43
3.1 Introduction	43
3.2 Abutment Configuration.....	44
3.3 Steel Material Modeling.....	45
3.3.1 Tension Test.....	48
3.3.2 Validity of Constant Volume Assumption	52

3.3.3	Elastic Rebound of Cross Section.....	54
3.3.4	Proposing a New Model for Steel Material	56
3.3.5	Curve Fitting.....	57
3.3.6	A Sample Model: Grade 50 Steel	61
3.4	Concrete Material Modeling.....	61
3.4.1	Concrete Response under Compression	62
3.4.2	Concrete Compression Response Models	63
A)	Hognestad Model	64
B)	Polynomial Model.....	65
C)	Carreira and Chu Model.....	65
D)	Comparison of Different Models	66
3.4.3	Concrete Response under Tension.....	67
3.4.4	Concrete Tension Response Models.....	68
3.4.5	Concrete Response Modeling in Abaqus.....	72
3.4.6	Employed Concrete Models	77
3.5	Elements	79
3.5.1	C3D8(R)	79
3.5.2	C3D4.....	81
3.5.3	C3D10M	81
3.5.4	T3D2	81
3.6	Stress Functions Definition	81
3.7	Finite Element Modeling.....	85
3.8	Moment Capacity of the Superstructure.....	98
3.9	Results of Analysis.....	101
3.9.1	Girder Stresses	101
A)	Effect of Girder Stiffener	102
B)	Effect of Girder End Shear Studs.....	103
C)	Ultimate Loading	104
3.9.2	Abutment Wall Stresses At Girder Embedment Zone.....	108
A)	Effect of Girder Stiffener on Wall Stresses	109
B)	Effect of Girder End Shear Studs.....	110
3.9.3	Abutment Wall Stresses At Pile Embedment Zone	112
A)	Effect of Pile Stiffener	113
3.9.4	Pile Stresses	114
A)	Effect of Pile Stiffener	115
Chapter 4	Effect of Curvature on Steel IA Bridges.....	117
4.1	Introduction	117
4.2	Bridge Configuration.....	118
4.2.1	Superstructure	118
4.2.2	Abutments.....	120
4.2.3	Piers	121
4.3	Finite Element Modeling.....	123

4.3.1	Material Properties.....	123
4.3.2	Loading.....	124
A)	Dead Load (DC).....	124
B)	Wearing Surface Load (DW).....	124
C)	Earth Pressure (EH).....	125
D)	Live Load (LL).....	126
E)	Braking Force (BR).....	127
F)	Centrifugal Force (CE).....	128
G)	Wind Load (WS).....	129
H)	Uniform Temperature Changes (TU).....	133
I)	Temperature Gradient (TG).....	133
J)	Shrinkage (SH).....	134
4.3.3	Load Combinations.....	138
4.3.4	Soil-Structure Interaction.....	140
A)	Soil-Abutment interaction.....	141
B)	Soil-Pile Interaction.....	143
B1)	Lateral Load-Deflection in Soft Clay.....	144
B2)	Lateral Load-Deflection in Sand.....	147
4.3.5	Elements.....	152
A)	Beam Element.....	153
B)	Shell Element.....	153
C)	Nonlinear Link Element.....	154
D)	Nonlinear Support Element.....	157
4.3.6	Finite Element Models.....	158
4.4	Results of FE Analysis.....	159
4.4.1	Effects of Length and Curvature on Load Responses.....	161
A)	Bending Moment of Abutment Piles.....	163
A1)	Contraction.....	164
A2)	Expansion.....	166
A3)	Live Load.....	168
A4)	Wind Load.....	170
A5)	Dead Load.....	172
A6)	Concrete Shrinkage.....	174
A7)	Horizontal Earth Pressure.....	176
A8)	Centrifugal Force.....	178
A9)	Weight of Wearing Surface.....	179
A10)	Braking Force.....	181
A11)	Positive Temperature Gradient.....	183
A12)	Negative Temperature Gradient.....	185

A13) Combination of the Loads	186
B) Shear Force of Abutment Piles	188
4.4.2 Bridge Movement	189
A) Factors Affecting Bridge Displacement.....	192
B) Bridge Shortening Due to Contraction	196
C) Bridge Shortening Due to Shrinkage	198
D) Total Bridge Shortening.....	199
E) Effect of Bridge Width on the Displacement Direction.....	201
E1) Effect of Width on Contraction End Displacement	201
E2) Effect of width on Shrinkage End Displacement	202
E3) Effect of width on Total End Displacement	202
F) Direction of Displacement	204
G) Bridge End Displacement	207
H) Step by Step Procedure	211
I) Example	212
4.4.3 Pile Orientation.....	214
A) Analysis of Modeled Bridges with Weak and Strong Orientation for Abutment Piles.....	217
B) The Procedure to Find the Optimal Pile Orientation	228
4.4.4 Effect of Bearing Type and Orientation	228
A) Effect of Bearing Type on Abutment Pile Moments	230
B) Effect of Bearing Type on Pier Columns Moments.....	231
C) Bearing Orientation.....	234
Chapter 5 Effect of Curvature on Concrete IA Bridges.....	237
5.1 Introduction	237
5.2 Bridge Configuration.....	238
5.2.1 Superstructure	238
5.2.2 Abutments.....	239
5.2.3 Piers	240
5.3 Finite Element Modeling.....	242
5.3.1 Material Properties.....	242
5.3.2 Loading	242
A) Dead Load (DC).....	243
B) Wearing Surface Load (DW)	243
C) Earth Pressure (EH)	244
D) Live Load (LL)	244
E) Braking Force (BR).....	246
F) Centrifugal Force (CE)	246
G) Uniform Temperature Changes.....	247
H) Temperature Gradient	248
I) Shrinkage	248
5.3.3 Soil-Structure Interaction.....	250

A)	Soil-Abutment Interaction	250
B)	Soil-Pile Interaction	251
B1)	Lateral Load-Deflection in Soft Clay	251
B2)	Lateral Load-Deflection in Sand.....	254
5.3.4	Elements	257
A)	Beam Element.....	258
B)	Shell Element.....	258
C)	Nonlinear Link Element.....	258
D)	Nonlinear Support Element.....	260
5.3.5	Finite Element Models.....	261
5.4	Results of FE Analysis	262
5.4.1	Effect of Length and curvature on Load Responses.....	263
A)	Bending Moment of Abutment Piles	263
A1)	Contraction.....	263
A2)	Expansion.....	264
A3)	Live Load	265
A4)	Dead Load	266
A5)	Concrete Shrinkage.....	267
A6)	Horizontal Earth Pressure (EH).....	268
A7)	Centrifugal Force	271
A8)	Weight of Wearing Surface	272
A9)	Braking Force.....	273
A10)	Positive Temperature Gradient	274
A11)	Negative Temperature Gradient	275
A12)	Combination of the Loads	276
B)	Shear Forces of Abutment Piles.....	277
5.4.2	Pile Orientation.....	277
5.4.3	Bearing-Isolated Pier vs. Flexible Integral Pier.....	284
5.4.4	Mitigation of Moment Magnification.....	285
Chapter 6	Concluding Remarks and Future Research	291
6.1	Connections of Integral Abutments.....	291
6.2	Steel I-girder IA Bridges	292
6.2.1	Effects of Bridge length and curvature on load responses	293
6.2.2	Bridge Movement	295
6.2.3	Pile Orientation.....	297
6.2.4	Effect of Bearing Type	297
6.3	Concrete IA Bridges.....	299

6.3.1 Effects of Bridge Length and Curvature on Load Responses.....	299
6.3.2 Pile Orientation.....	301
6.3.3 Bearing-Isolated Piers versus Integral Piers.....	302
6.3.4 Mitigation of Moment Magnification Factor.....	302
6.4 Recommendations for Future Research.....	303

References 305

Appendix A Effect of Bridge Length and Curvature on Shear Force of Abutment Piles 317

A1 Contraction	317
A2 Expansion	319
A3 Live Load.....	321
A4 Wind Load	323
A5 Dead Load	325
A6 Concrete Shrinkage	327
A7 Horizontal Earth Pressure.....	329
A8 Centrifugal Force.....	331
A9 Weight of Wearing Surface	332
A10 Braking Force	334
A11 Positive Temperature Gradient.....	336
A12 Negative Temperature Gradient	337
A13 Combination of the Loads	339

Appendix B MATLAB Moment-curvature Program.....342-351

List of Figures

Figure 1.1-1	A Typical a) Integral Abutment b) Semi-Integral Abutment.....	4
Figure 2.2-1	Plan View of a Curved Girder Under Gravity Loads in z Direction.....	24
Figure 2.3-1	Effect of Torsional Moment Applied to a Cantilever I-Girder (Boresi et al.).....	28
Figure 2.4-1	Plan View of the Two-Girder Bridge.....	33
Figure 2.4-2	Flange Forces of the Girders.....	33
Figure 2.4-3	Equilibrium of a Segment of the Girder Flange.....	34
Figure 2.4-4	3D View of the Two-Girder Bridge.....	35
Figure 2.5-1	Cross Section of the Multi-Girder Curved Bridge.....	37
Figure 3.2-1	General Configuration of an Integral Abutment.....	45
Figure 3.3-1	Strain-Stress Curves (Tension Test # 1)	49
Figure 3.3-2	Strain-Stress Curves (Tension Test # 2)	50
Figure 3.3-3	Strain-Stress Curves (Tension Test # 3)	50
Figure 3.3-4	Strain-Stress Curves (Tension Test # 4)	51
Figure 3.3-5	Designation of the Key Points on the Strain-Stress Curves.....	56
Figure 3.3-6	Scheme of Fitted Curve Between Points C and D	59
Figure 3.3-7	Fitted Curve Between Points C and D	60
Figure 3.3-8	Material Model for Grade 50 Steel	61
Figure 3.4-1	Strain-Stress Curves of Concrete Specimens of Different Strength	63
Figure 3.4-2	Hognestad Model for Strain-Stress of Concrete in Compression	64
Figure 3.4-3	Strain-Stress Curves of Concrete in Compression in Different Models	67
Figure 3.4-4	Strain-Stress Curves of Concrete in Tension in Different Models	68
Figure 3.4-5	Schematic Strain-Stress Curve of Concrete in Tension	69
Figure 3.4-6	Strain-Stress Curves of Concrete in Tension and Compression	73
Figure 3.4-7	Biaxial Failure Surface of Concrete.....	75

Figure 3.4-8	Definition of Compressive Inelastic strain Used for Definition of Compression Hardening Data (Abaqus Documentation).....	76
Figure 3.4-9	Definition of Tensile Cracking Strain used for Definition of Tension Stiffening Data	77
Figure 3.4-10	Total and Inelastic Strain vs. Stress for 4 ksi Concrete in Compression ...	78
Figure 3.4-11	Total and Cracking Strain vs. Stress for 4 ksi Concrete in Tension	79
Figure 3.5-1	C3D8 Brick Element.....	80
Figure 3.5-2	C3D10M Tetrahedron Element.....	81
Figure 3.7-1	General Configuration of the Integral Abutment Model.....	86
Figure 3.7-2	Girder Element of the Connection	87
Figure 3.7-3	Embedded End of the Girder.....	88
Figure 3.7-4	Highlighted Position of the Girder Element.....	89
Figure 3.7-5	Concrete Wall of the Abutment	90
Figure 3.7-6	Highlighted Position of the Abutment Wall.....	91
Figure 3.7-7	Highlighted Position of the Deck Slab.....	92
Figure 3.7-8	Haunch Element of the Connection	92
Figure 3.7-9	Highlighted Position of the Haunch.....	93
Figure 3.7-10	H- Pile Element of the Connection	94
Figure 3.7-11	Highlighted Position of the H-Piles	95
Figure 3.7-12	Highlighted Rebars of the Deck Slab.....	96
Figure 3.7-13	Highlighted Rebars of the Abutment Wall	97
Figure 3.7-14	Shear Studs Attached to the Girder Bottom Flange	97
Figure 3.8-1	Superstructure Section at the Vicinity of Abutment Wall (Section A-A)	99
Figure 3.8-2	Moment–Curvature of the Superstructure at Section A-A.....	99
Figure 3.8-3	Stress Distribution in the Superstructure at the Vicinity of Abutment Wall Corresponding to Maximum Moment Capacity.....	100
Figure 3.9-1	Mises Stresses of Girder without any Stiffener or End Shear Stud	102
Figure 3.9-2	Mises Stresses of Girder with Stiffener without End Shear Stud	103
Figure 3.9-3	Mises Stresses of Girder with End Shear Stud without Stiffener	104
Figure 3.9-4	Mises Stresses of Half of the Girder	105
Figure 3.9-5	Mises Stresses of Concrete Deck	106
Figure 3.9-6	Mises Stresses of Girder’s End Corresponding to Mid-span Plastification.....	107

Figure 3.9-7	Mises Stresses of Abutment Wall Supporting an Unstiffened Girder	108
Figure 3.9-8	Mises Stresses of Abutment Wall Supporting a Stiffened Girder.....	109
Figure 3.9-9	Mises Stresses of Abutment Wall - Girder without End Shear Studs.....	111
Figure 3.9-10	Mises Stresses of Abutment Wall - Girder with End Shear Studs.....	111
Figure 3.9-11	Mises Stresses of the Concrete Wall Around the Piles	112
Figure 3.9-12	Stiffened Pile at the Wall Lower Face Section	113
Figure 3.9-13	Mises Stresses of the Concrete Wall Around the Piles with Stiffener.....	114
Figure 3.9-14	Mises Stresses of Abutment Piles	115
Figure 3.9-15	Mises Stresses of Abutment Piles with Stiffener	116
Figure 4.1-1	A Curved Steel I-girder Bridge Similar to the Studied Bridges	118
Figure 4.2-1	Cross Section of the Composite Steel Superstructure.....	119
Figure 4.2-2	Steel I-Girder Dimensions	120
Figure 4.2-3	Abutment Integral Details.....	121
Figure 4.2-4	Pier Configuration.....	122
Figure 4.2-5	Springs Modeling the Bearings of the Piers.....	123
Figure 4.3-1	Wearing Surface of the Modeled Steel Bridges.....	125
Figure 4.3-2	Positioning of the Live Load.....	127
Figure 4.3-3	Positive Temperature Gradient in the Superstructure Section	134
Figure 4.3-4	Annual Average Ambient Relative Humidity in Percent.....	136
Figure 4.3-5	Relationship of Wall Movement vs. Soil Pressure.....	142
Figure 4.3-6	Force-Displacement Curves of the Abutment Backfill Springs.....	143
Figure 4.3-7	Force-Displacement Curves of the Springs of Piles of Abutments in Soft Clay	146
Figure 4.3-8	Force-Displacement Curves of the Springs of Piles of Piers in Soft Clay	147
Figure 4.3-9	Initial Modulus of Subgrade Reaction	148
Figure 4.3-10	Values of Coefficients C_1 , C_2 and C_3 as a Function of Angle of Friction	150
Figure 4.3-11	Force-Displacement Curves of the Springs of Piles of Piers in Sand.....	151
Figure 4.3-12	Force-Displacement Curves of the Springs of Piles of Abutments in Sand.....	152
Figure 4.3-13	Modeling of the Bearings.....	156
Figure 4.3-14	Typical Finite Element Models of the Studied Bridges	159

Figure 4.4-1	Maximum Moment in Abutment Piles Due to Contraction	165
Figure 4.4-2	Normalized Moment in Abutment Piles Due to Contraction.....	166
Figure 4.4-3	Maximum Moment in Abutment Piles Due to Expansion	167
Figure 4.4-4	Normalized Moment in Abutment Piles Due to Expansion.....	168
Figure 4.4-5	Maximum Moment in Abutment Piles Due to Live Load	169
Figure 4.4-6	Normalized Moment in Abutment Piles Due to Live Load	170
Figure 4.4-7	Maximum Moment in Abutment Piles Due to Wind Load.....	171
Figure 4.4-8	Normalized Moment in Abutment Piles Due to Wind Load.....	172
Figure 4.4-9	Maximum Moment in Abutment Piles Due to Dead Load	173
Figure 4.4-10	Normalized Moment in Abutment Piles Due to Dead Load	174
Figure 4.4-11	Maximum Moment in Abutment Piles Due to Concrete Shrinkage	175
Figure 4.4-12	Normalized Moment in Abutment Piles Due to Concrete Shrinkage	176
Figure 4.4-13	Maximum Moment in Abutment Piles Due to Horizontal Earth Pressure	177
Figure 4.4-14	Normalized Moment in Abutment Piles Due to Horizontal Earth Pressure	178
Figure 4.4-15	Maximum Moment in Abutment Piles Due to Centrifugal Force.....	179
Figure 4.4-16	Maximum Moment in Abutment Piles Due to Weight of wearing Surface	180
Figure 4.4-17	Normalized Moment in Abutment Piles Due to Weight of Wearing Surface	181
Figure 4.4-18	Maximum Moment in Abutment Piles Due to Braking Force.....	182
Figure 4.4-19	Normalized Moment in Abutment Piles Due to Braking Force.....	183
Figure 4.4-20	Maximum Moment in Abutment Piles Due to Positive Temperature Gradient.....	184
Figure 4.4-21	Normalized Moment in Abutment Piles Due to Positive Temperature Gradient.....	184
Figure 4.4-22	Maximum Moment in Abutment Piles Due to Negative Temperature Gradient.....	185
Figure 4.4-23	Normalized Moment in Abutment Piles Due to Negative Temperature Gradient.....	186
Figure 4.4-24	Maximum Moment in Abutment Piles in Load Combinations Envelope..	187
Figure 4.4-25	Normalized Moment in Abutment Piles in Load Combinations Envelope.....	188
Figure 4.4-26	Directions of End Displacements in A Curved Bridge	193

Figure 4.4-27	Bridge End Movement- a) Pure Translation -b) Rotation.....	194
Figure 4.4-28	Modification Factor for Bridge Shortening Due to Contraction.....	197
Figure 4.4-29	Modification Factor for Bridge Shortening Due to Shrinkage.....	199
Figure 4.4-30	Modification Factor for Bridge Shortening Applied to Total Shortening	200
Figure 4.4-31	Direction of End Displacement Due to Contraction	201
Figure 4.4-32	Angles α_{in} and α_{out} in Total Displacement.....	203
Figure 4.4-33	Values $\alpha/90^\circ$ versus W/Lc	204
Figure 4.4-34	Modified Displacement Directions versus L/R.....	207
Figure 4.4-35	Deformed Bridge General Configuration.....	208
Figure 4.4-36	Deformed Bridge Simplified Configuration	209
Figure 4.4-37	Critical Load Combination Type	215
Figure 4.4-38	Critical Load Combination Type	223
Figure 4.4-39	Direction of Pile Displacement.....	224
Figure 4.4-40	Average Displacement Directions of Abutment Piles Top Node	225
Figure 4.4-41	Design Displacement Directions of Abutment Piles Top Node	226
Figure 4.4-42	Bending Moment of Abutment Piles with Different Bearing Types	230
Figure 4.4-43	Longitudinal Bending Moment of Pier Columns with Different Bearing Types	232
Figure 4.4-44	Transverse Bending Moment of Pier Columns with Different Bearing Types.....	234
Figure 4.4-45	Guided Bearing Orientation in Jointed Bridges	235
Figure 4.4-46	Guided Bearing Orientation in Bridges with Restraint Superstructure for a Trial Point	236
Figure 5.1-1	A Concrete Curved Integral Bridge Similar to the Studied Bridges.....	238
Figure 5.2-1	Cross Section of the Superstructure of the Modeled Bridges	239
Figure 5.2-2	A Typical Integral Connection for Voided Slab Bridges.....	240
Figure 5.2-3	Integral Connection of Piers and Superstructure	241
Figure 5.2-4	Bearing-Isolated Connection of Piers and Superstructure	241
Figure 5.3-1	Wearing Surface of the Modeled Bridges.....	244
Figure 5.3-2	Positioning of the Live Load (distances in inch)	245
Figure 5.3-3	Force-Displacement Curves of the Abutment Backfill Springs.....	251
Figure 5.3-4	Force-Displacement Curves of the Springs of Piles of Abutments in Soft Clay	253

Figure 5.3-5	Force-Displacement Curves of the Springs of Piles of Piers in Soft Clay	254
Figure 5.3-6	Force-Displacement Curves of the Springs of Piles of Piers in Sand	256
Figure 5.3-7	Force-Displacement Curves of the Springs of Piles of Abutments in Sand	257
Figure 5.3-8	Modeling of the Elastomeric Bearings	260
Figure 5.3-9	A Typical Finite Element Model of the Studied Bridges	262
Figure 5.4-1	Maximum Moment of Abutment Piles Due to Contraction	264
Figure 5.4-2	Maximum Moment of Abutment Piles Due to Expansion	265
Figure 5.4-3	Maximum Moment of Abutment Piles Due to Live Load	266
Figure 5.4-4	Maximum Moment of Abutment Piles Due to Dead Load	267
Figure 5.4-5	Maximum Moment of Abutment Piles Due to Concrete Shrinkage	268
Figure 5.4-6	Maximum Moment of Abutment Piles Due to Horizontal Earth Pressure	269
Figure 5.4-7	Plan View of Deformed Shape of the Bridge with R=200' under EH	270
Figure 5.4-8	Plan View of Deformed Shape of the Bridge with R=600' under EH	270
Figure 5.4-9	Plan View of Deformed Shape of the Bridge with R=1000' under EH	271
Figure 5.4-10	Maximum Moment of Abutment Piles Due to Centrifugal Force	272
Figure 5.4-11	Maximum Moment of Abutment Piles Due to Weight of Wearing Surface	273
Figure 5.4-12	Maximum Moment of Abutment Piles Due to Braking Force	274
Figure 5.4-13	Maximum Moment of Abutment Piles Due to Positive Temperature Gradient	275
Figure 5.4-14	Maximum Moment of Abutment Piles Due to Negative Temperature Gradient	276
Figure 5.4-15	Envelope of Moment of Abutment Piles in Different Load Combinations	277
Figure 5.4-16	PCACOL Design Sheet for Strong-Axis Orientation (1 of 2)	280
Figure 5.4-17	PCACOL Design Sheet for Strong-Axis Orientation (2 of 2)	281
Figure 5.4-18	PCACOL Design Sheet for Weak-Axis Orientation (1 of 2)	282
Figure 5.4-19	PCACOL Design Sheet for Weak-Axis Orientation (2 of 2)	283
Figure 5.4-20	Envelope of Moment of Abutment Piles in Different Load Combinations in Bridges with Integral Piers vs. Bridges with Elastomeric Isolated Piers	285
Figure 5.4-21	$P\Delta$ Effect in Non-Sway Mode	288

Figure 5.4-22	$P\Delta$ Effect in Sway Mode	288
Figure 5.4-23	$P\Delta$ Effect in a Jointed Abutment Bridge with Flexible Piers	289
Figure 5.4-24	$P\Delta$ Effect in an Integral Abutment Bridge with Flexible Piers	290
Figure A-1.	Maximum Shear Force in Abutment Piles Due to Contraction	318
Figure A-2.	Normalized Shear Force in Abutment Piles Due to Contraction	319
Figure A-3.	Maximum Shear Force in Abutment Piles Due to Expansion	320
Figure A-4.	Normalized Shear Force in Abutment Piles Due to Expansion	321
Figure A-5.	Maximum Shear Force in Abutment Piles Due to Live Load.....	322
Figure A-6.	Normalized Shear Force in Abutment Piles Due to Live Load	323
Figure A-7.	Maximum Shear Force in Abutment Piles Due to Wind Load	324
Figure A-8.	Normalized Shear Force in Abutment Piles Due to Wind Load.....	325
Figure A-9.	Maximum Shear Force in Abutment Piles Due to Dead Load.....	326
Figure A-10.	Normalized Shear Force in Abutment Piles Due to Dead Load	327
Figure A-11.	Maximum Shear Force in Abutment Piles Due to Concrete Shrinkage.....	328
Figure A-12.	Normalized Shear Force in Abutment Piles Due to Concrete Shrinkage ..	329
Figure A-13.	Maximum Shear Force in Abutment Piles Due to Earth Pressure	330
Figure A-14.	Normalized Shear Force in Abutment Piles Due to Earth Pressure.....	331
Figure A-15.	Maximum Shear Force in Abutment Piles Due to Centrifugal Force	332
Figure A-16.	Maximum Shear Force in Abutment Piles Due to Weight of Wearing Surface	333
Figure A-17.	Normalized Shear Force in Abutment Piles Due to Weight of Wearing Surface	334
Figure A-18.	Maximum Shear Force in Abutment Piles Due to Braking Force	335
Figure A-19.	Normalized Shear Force in Abutment Piles Due to Braking Force	335
Figure A-20.	Maximum Shear Force in Abutment Piles Due to Positive Temperature Gradient.....	336
Figure A-21.	Normalized Shear Force in Abutment Piles Due to Positive Temperature Gradient	337
Figure A-22.	Maximum Shear Force in Abutment Piles Due to Negative Temperature Gradient	338
Figure A-23.	Normalized Shear Force in Abutment Piles Due to Negative Temperature Gradient	339
Figure A-24.	Envelope of Maximum Shear Force in Abutment Piles in Different Load Combinations	340

Figure A-25. Envelope of Normalized Shear Force in Abutment Piles in Different Load Combinations

PREVIEW

List of Tables

Table 1.1-1	Different Types of Jointless Bridges.....	3
Table 2.5-1	C Factor for Different Number of Girders	40
Table 4.3-1	Multilane Presence Factors	126
Table 4.3-2	C Factor for Different Radii.....	129
Table 4.3-3	Base Wind Pressure, P_B Corresponding to $V_B = 100 \text{ mph}$	130
Table 4.3-4	Values of V_o and Z_o for Various Upstream Conditions	131
Table 4.3-5	Base Wind Pressure, P_B for Various Angles of Attack.....	131
Table 4.3-6	Wind Pressure, P_D for Various Angles of Attack.....	133
Table 4.3-7	Proposed API p-y Curve for Soft Clay	144
Table 4.4-1	Normalized Weight Factors for Bending Moment of Abutment Piles	163
Table 4.4-2	Direction of Total Displacement (Results of FE Analyses) (W=60'-8")...	202
Table 4.4-3	Angles and Modified Angles of Total Displacement Direction.....	206
Table 4.4-4	Ratios of Abutment Pile Stresses (Weak Axis Orientation to Strong Axis Orientation).....	219
Table 4.4-5	Critical Load Combination Category for Abutment Pile Stresses	222
Table 4.4-6	Ratio of Piles Weak-axis Orientation Stress to Optimized Stress	227
Table 4.4-7	Ratio of Piles Strong-axis Orientation Stress to Optimized Stress	227
Table 4.4-8	Different Bearing Types and the Associated DOF's.....	229
Table 5.3-1	C Factor for Different Radii	247
Table 5.4-1	Internal Forces of Shafts with Different Orientations.....	278

Chapter 1

Introduction, Background and Objectives

1.1 Introduction

Bridges have been built since thousands of years ago by human beings. From prehistoric times to the Renaissance bridges had two main characteristics: The main construction materials were stone and natural cement and the spans were less than 100 feet. Despite the limitations that the architects and engineers of those times had, long bridges up to a total length of 1000 feet can be found among ancient bridges. After the Renaissance, modern bridges came into existence in the seventeen and eighteen century. The greatest differences of these modern bridges and the old ones were the material and span length. The material changed to iron (or steel) and later concrete. The span length gradually increased up to 3000 feet in the early twentieth century. So, the engineers were in charge of designing longer and longer bridges.

This trend in building bridges caused new approaches to appear in bridge industry. To accommodate the movements of long bridges, the designers adopted new techniques in

their designs. Moveable expansion joints and bearings were among those techniques. These devices have been used in bridges for more than 200 years. But, their performance has affected the long term performance of new bridges. Expansion joints have different designs which all of them have some sort of dysfunction. Even though they have high quality in the first months or years of service, after a longer time, most of them have problems such as leakage and poor ride quality due to wear or fracture. Bearings also have shown their intrinsic problems. In most bearing types, elastomeric layers are used. These elastomers lose their original properties in time. Ozone can damage the elastomer, even if there is no load or movement applied to the elastomer. That's why most of bearings should be replaced after some years.

The deficiencies of expansion joints and bearings drew the bridge designers to some new concepts of bridge design in the past years. Elimination of joints and bearings was the new target. This led to the introduction of a new type of structural system known as *Integral bridges*. These bridges are composed of:

- Abutments at the two ends
- Approach slabs that rest on abutments and their backfill
- Intermediate piers
- And finally a “*jointless*” superstructures built integral to the abutments

Note that there are no joints from the end to end of approach slabs. This bridge system is an ideal one which has no bearings and no joints. But, based on the needs, some other structural systems have also developed. First is an integral bridge that has rigid piers with movable and/or fixed bearings. In this type of bridges, all expansion joints and also the

abutment bearings are eliminated. But, piers still have bearings. If the durability of the pier bearings is guaranteed, these bridges can survive for a long time without any major deterioration. The second system has integral piers, but it has bearings in the abutments. In these bridges, the piers are flexible to be able to accommodate the movements, and the abutments are rigid, so they are isolated from the superstructure. A third system is a jointless bridge that has bearings both in the abutments and piers. Those jointless structural systems that have bearings in the abutments are called *semi-integral*. Table 1.1-1 shows different types of jointless bridges.

Bridge Type	Joint	Bearing over Piers	Bearing in Abutments
Integral with Flexible Piers	No	No	No
Integral with Rigid Piers	No	Yes	No
Semi-integral with Flexible Piers	No	No	Yes
Semi-integral with Rigid Piers	No	Yes	Yes

Table 1.1-1. Different Types of Jointless Bridges

As described before, integral bridges do not have joints whether they have bearings or not. Therefore, a better name for these bridges is “Jointless”. These two terms have been used interchangeably in the literature and also in the present study. The integral bridges with no bearing in the abutments are sometimes called “integral Abutment Bridges”. Figure 1.1-1 illustrates typical integral and semi-integral abutment details.